

Stress orientation to 5 km depth in the basement below Basel (Switzerland) from borehole failure analysis

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ABSTRACT

A vertical profile of maximum horizontal principal stress, SHmax, orientation to 5 km depth was obtained beneath the Swiss city of Basel from observations of wellbore failure derived from ultrasonic televiewer images obtained in two 1 km distant near-vertical boreholes: a 2755 m exploration well (OT2) imaged from 2550 m to 2753 m across the granitic basement-sediment interface at 2649 m; and a 5 km deep borehole (BS1) imaged entirely within the granite from 2569 m to 4992 m. Stress-related wellbore failure in the form of breakouts or drilling-induced tension fractures (DITFs) occurs throughout the depth range of the logs with breakouts predominant. Within the granite, DITFs are intermittently present, and breakouts more or less continuously present over all but the uppermost 100 m where they are sparse. The mean SHmax orientations from DITFs is $151 \pm 13^\circ$ whereas breakouts yield $143 \pm 14^\circ$, the combined value weighted for frequency of occurrence being $N144^\circ E \pm 14^\circ$. No marked depth dependence in mean SHmax orientation averaged over several hundred meters depth intervals is evident. This mean SHmax orientation for the granite is consistent with the results of the inversion of populations of focal mechanism solutions of earthquakes occurring between depths of 10–15 km within regions immediately to the north and south

of Basel, and with the T-axis of events occurring within the reservoir (Deichmann and Ernst, this volume). DITFs and breakouts identified in OT2 above and below the sediment-basement interface suggest that a change in SHmax orientation to $N115^\circ E \pm 12^\circ$ within the Rotliegendes sandstone occurs near its interface with the basement. The origin of the 20–30° change is uncertain, as is its lateral extent. The logs do not extend higher than 80 m above the interface, and so the data do not define whether a further change in stress orientation occurs at the evaporites. Near-surface measurements taken within 50 km of Basel suggest a mean orientation of N–S, albeit with large variability, as do the orientation of hydrofractures at depths up to 850 m within and above the evaporite layers and an active salt diapir, also within 50 km of Basel. Thus, the available evidence supports the notion that the orientation of SHmax above the evaporites is on average more N–S oriented and thus differs from the NW–SE inferred for the basement from the BS1/OS2 wellbore failure data and the earthquake data. Changes in stress orientation with depth can have significant practical consequences for the development of an EGS reservoir, and serve to emphasise the importance of obtaining estimates from within the target rock mass.

Introduction

Recently, two deep boreholes were drilled below the city of Basel in Switzerland with a view to developing an Enhanced/Engineered Geothermal System (EGS) within granitic basement rocks at 5 km depth (Häring et al. 2008). The first hole, OT2, was a vertical exploration hole drilled in 2001 to 2755 m depth (NB: all depths in this paper are given along hole from ground level) in order to establish the temperature gradient and the depth of basement. The bottom hole temperature was 123 °C and the basement was intersected at 2649 m. The second hole, BS1, was located about 1 km northwest of OT2 and was drilled vertically in 2006 to 5000 m with an 7–5/8 inch ID casing shoe set at 4629 m. Granite was encountered at 2426 m.

This hole was to be the first of a duplet that would constitute an EGS and allow heat to be extracted from the deep basement. The engineering of such systems benefits from knowledge of the stress state, and in particular, the orientation of the maximum principal horizontal stress, SHmax. Thus ultrasonic televiewer logs were run in both holes to attempt to image wellbore failure features that are stress orientation indicators. Subsequent events at the site provided further impetus to determine the stress state within the granite because a magnitude 3.4 seismic event occurred close to the well. The event was almost certainly triggered by fluid injection into the open hole section that was intended to enhance the permeability of the geothermal reservoir (Pine & Batchelor 1984; Evans et al. 2005). In this paper we analyse wellbore failure observa-

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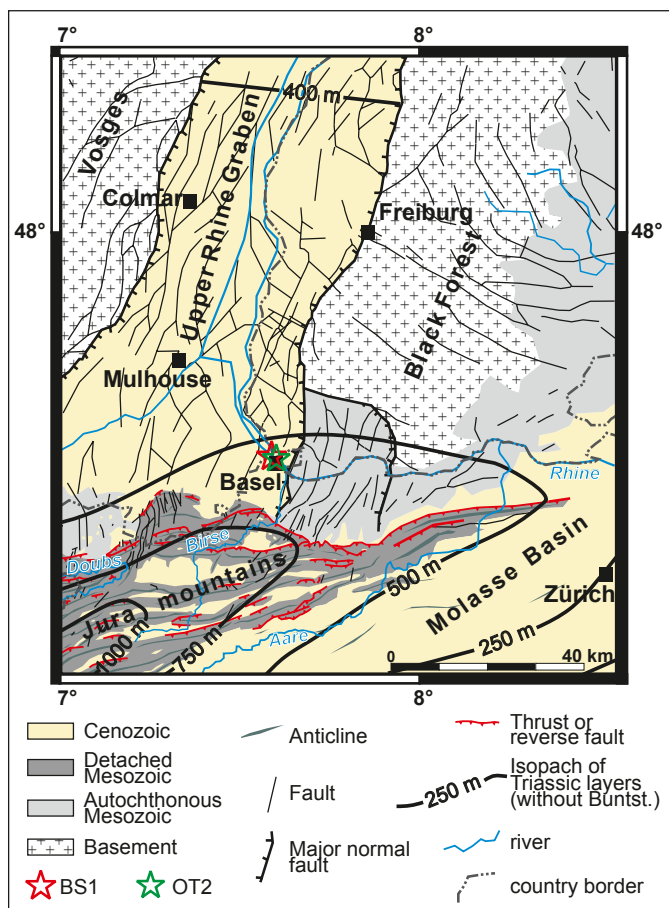


Fig. 1. Structural setting of the Basel boreholes. Note that the Triass beds are largely composed of evaporates layers. Compiled and modified after Illies (1972), Illies & Greiner (1979), Dezayes (1996), Sommaruga (1999) and Ustaszewski (2004).

tions in both OT2 and BS1 to determine the vertical profile of the orientation of the maximum horizontal principal stress, SHmax, throughout the depth range 2600–5000 m. The evaluation of the magnitude of stresses is beyond the scope of this paper. Both borehole breakouts and drilling-induced tension fractures (DITFs) are considered. The results are discussed within the context of regional stress defined by stress orientation measurements conducted in the area.

Geological setting

Basel is located at the southern end of the Upper Rhine Graben, where it meets the fold and thrust belt formed by the Jura mountains (see Fig. 1). In the vicinity of the wells, the basement surface is weathered (Häring 2001) and is overlain by about 2.5 km of sedimentary cover consisting of ~800 m of sandstones (Rotliegendes and Buntsandstein) which give way to limestones and marls that extend to close to the surface (Fig. 2). The latter contain a band of evaporite units that lie between 1.3 and 1.6 km depth. The lowermost is a 100–200 m thick halite

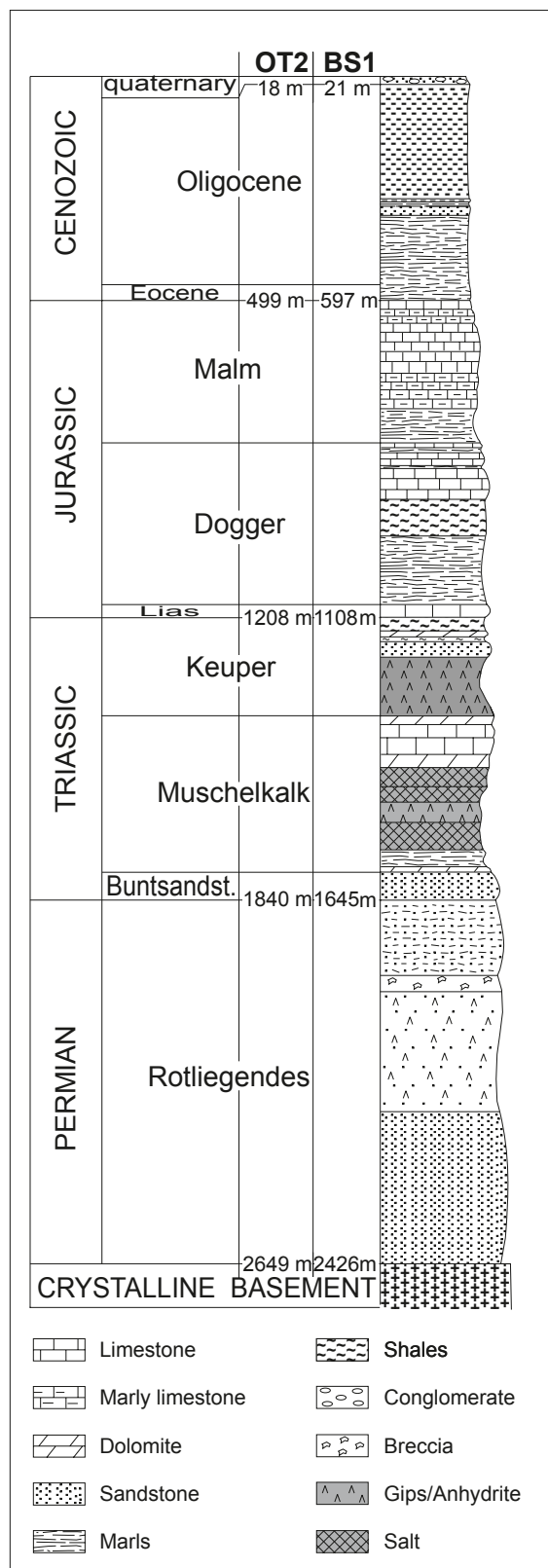


Fig. 2. Lithological section at OT2 and BS1 boreholes. All depths in this paper are given along hole from ground level. The wellhead of BS-1 is 250.2 m amsl, and that for OT-2 is 252.61 m amsl.

bed (Salzlager), and the uppermost is an anhydrite unit (Gipskeuper) of about 150 m thickness. The evaporite beds are extensive (e.g. Jordan 1994; Sommaruga 1999; Becker 2000) and constitute horizons of possible stress detachment (Becker et al. 1987; Evans and Roth 1998).

Log data and analysis methods

Log data

The log suite run in BS1 included a Schlumberger ultrasonic televiewer (UBI) which was run between 2569 and 4992 m shortly after drilling. The log is entirely in granite and begins 25 m inside the 10–3/4 inch casing. The open hole diameter below the 10–3/4 inch casing shoe is 9–7/8" to 4841 m depth and 8–1/2" below. The tool was operated at a frequency of 2.5 MHz and used a focused beam that would produce a spot size of about 0.64 cm in an 8–1/2" diameter borehole containing fresh water. The transponder rotated at 6 revolutions per second, and made 180 measurements per revolution, giving an angular resolution of 2° or 0.38 cm in an 8–1/2" hole. The log was acquired at a logging speed of 4 m/min giving a vertical resolution is 1.016 cm. Since the spot size is 0.635 cm, a small gap between measurements swathes is present.

The ultrasonic reflectivity and travel time logs acquired in the BS1 borehole are shown in Figs. 3a–b. An index of image quality based on usable image width as defined in Table 1 is shown in Fig. 3c. Approximately 89% of the log is considered to be of good quality. No strong key-seats were identified (e.g. Lofts & Bourke 1999). The images from the lowermost 250 m of borehole were ranked as poor quality, due to stick-slip movement of the sonde. This occurred every 0.5–1.0 m and involved sudden tool movements of 10–30 cm. Although this severely impacted the ability to recognise structures, it had substantially less effect on the breakout analysis. The only section of image that was totally unusable for this study is the uppermost 22 m of log where image orientation is affected by the casing.

The logs in OT2 were run by Baker-Atlas and included borehole wall microresistivity (STAR) and ultrasonic televiewer (CBIL). Only acoustic televiewer data were analysed. The televiewer was operated at a frequency of 250 KHz and had 250 samples per revolution. The logs were run from about 2550 to 2753 m depth and thus cover the lowermost 100 m of the Rotliegendes sandstone and the uppermost 100 m of basement (Fig. 4). Borehole diameter in this section is 6–1/4". The data were acquired in two runs which overlap between 2599 m and 2631 m. The lower section was logged first, and suffered problems with stick-slip movement of the sonde in the overlap interval. This occurs when the friction between the tool string and borehole wall is sufficient to bring the tool to a halt, and is promoted by wall irregularity, stiff centralisation, and depths sufficient to render the wireline system elastically compliant (Lofts & Bourke 1999). The problems were much reduced in the second log of this interval, which extended to the top of the logged section. Together, the two runs give a continuous log

Table 1. Ultrasonic borehole image quality classification.

designation	usable image width	examples
unusable	0%	in casing, log extremities
Poor quality	0–30%	Large caves, stick/slip, bit whirling
medium	30–70%	strong key-seat
Good	≥70%	–

of acceptable quality that extends through the basement-sediment interface (Fig. 4a–b). A more serious limitation of these data concerns the travel-time images which frequently show diametrically-opposite pairs of sectors where the travel time has a constant, unrealistically-small value that would imply a borehole diameter smaller than the bit size. These anomalous data show as white on the image of Fig. 4b, and occur predominantly between 2565 and 2635 m depth. They often occur in zones where the travel-time suggests spalling has occurred (i.e. the dark zones of Fig. 4b). Moreover, the anomalously low travel-times are invariably accompanied by a low signal amplitude (dark zones of Fig. 4a). These observations collectively suggest an instrumental origin for the anomalous data points, triggered by the absence of a detected reflection from the borehole wall within the listening window, such as can occur through non-normal incidence on an irregular borehole wall (e.g. Georgi 1985). Possible origins for the stable, anomalously-low travel times are that it was the ‘time-out’ setting chosen by the logging engineer, or that it represents the arrival time of the ‘boot reflection’. Henceforth we will assume that the anomalously-low travel times indicate spalling, an assumption that is supported both by the diametric symmetry of the zones expected for stress-induced breakouts, and their orthogonality to drilling-induced tension fractures. However, it is clear that the detailed geometry of the spall cannot be determined from the data.

Analysis methods

Two types of borehole failure are recognised as reliable stress indicators: drilling-induced tension fractures (DITFs) and breakouts. DITFs (e.g. Brudy & Zoback 1999) form where the tensile tangent stress component developed by the cooling of the borehole wall by the circulating drilling mud is sufficient to produce tensile failure at some point around the borehole. The net tangent stress around the borehole is the sum of the cooling component, which is axially-symmetric, and the natural wellbore stress concentration arising from the ‘far-field’ stresses, which is usually not axially-symmetric and may be everywhere compressive. In the case of a vertical borehole penetrating a medium in which one principal stress is also vertical, the least compressive value of the tangent stress variation about the borehole due to the far field stresses occurs in the direction of SHmax. Thus, it is in this direction that the greatest tension develops and a pair of diametrically-opposite, axial, drilling-induced tension fractures (A-DITFs) form. However,

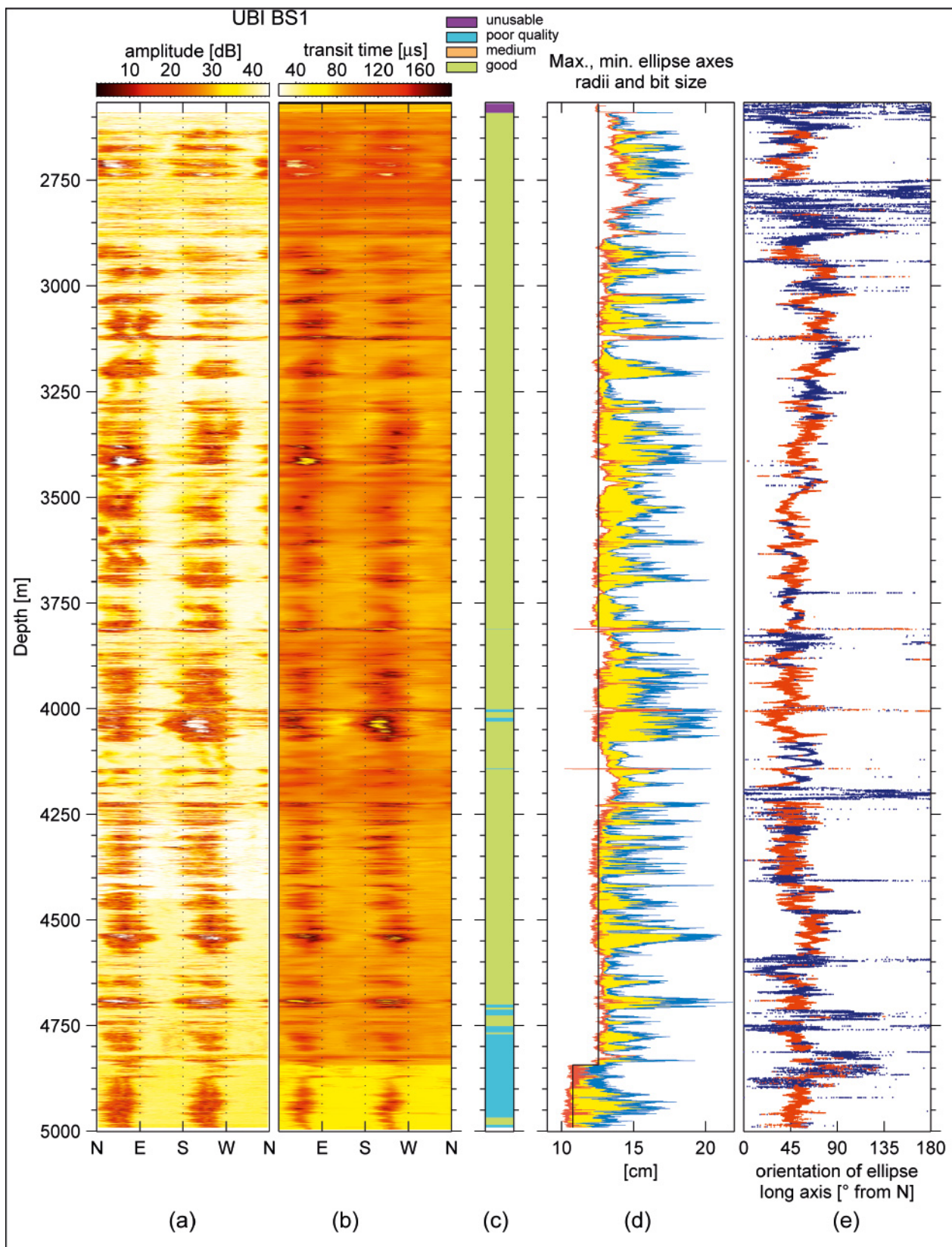


Fig. 3. Summary of UBI log from BS1: a) acoustic reflectivity log. b) transit time log. c) image quality rating. d) Bit size (black line), maximum and minimum ellipse radii. e) Orientation of ellipse long axis.

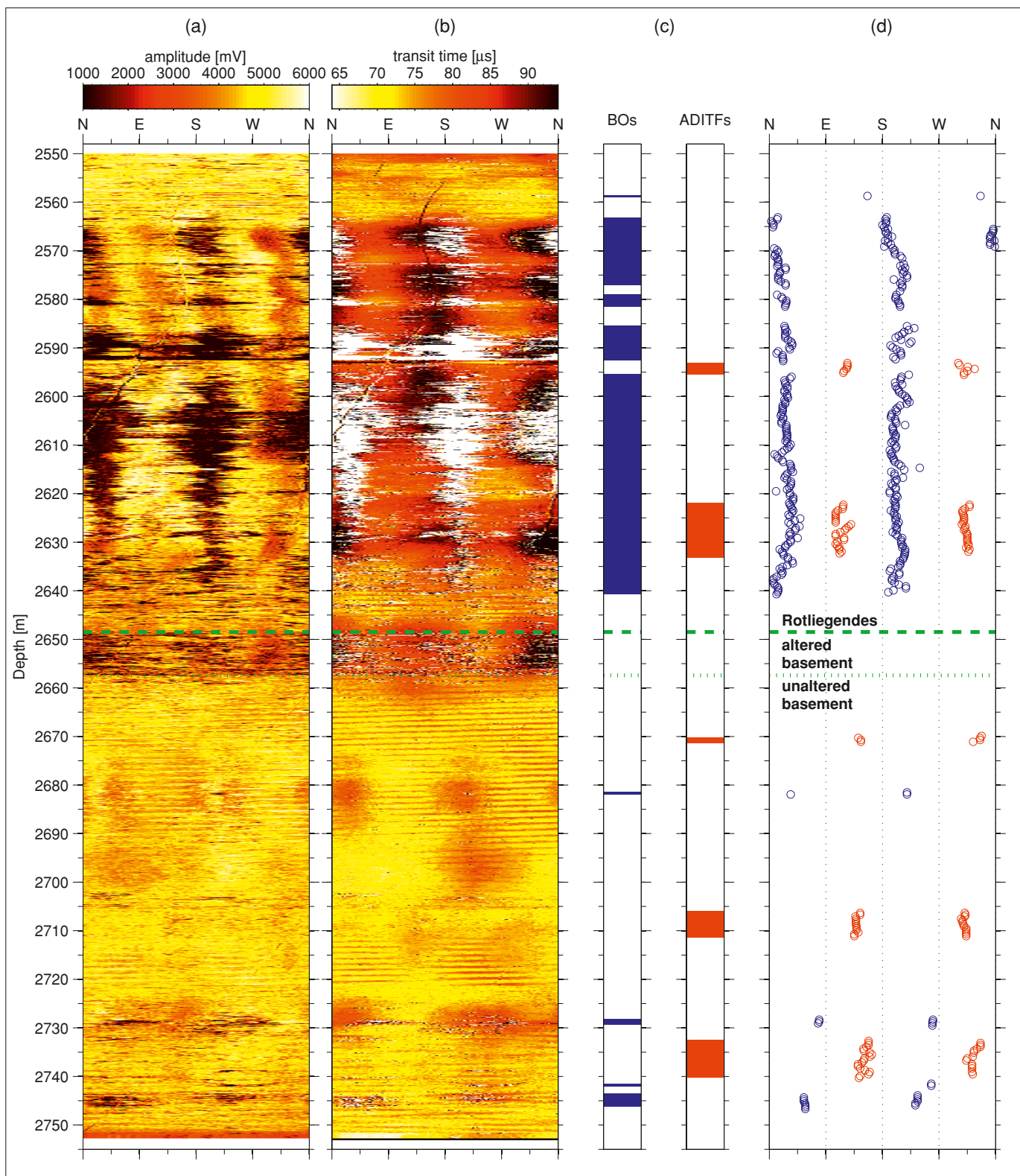


Fig. 4. Summary of the ultrasonic imaging (CBIL tool from Baker Atlas) and borehole failure in OT2 borehole. a) acoustic reflectivity log. b) transit time log c) breakouts (BOs) and axial drilling-induced fracture (A-DITFs) determination. d) orientation measurement of the breakouts (blue) and the axial drilling-induced tension fractures (red). The depth of the transition from the sedimentary cover (Rotliegende) to altered basement and unaltered basement are indicated by dashed green lines.

in the case where the borehole axis is not aligned with a principal stress axis, the criterion for tensile failure might still be met but then DITFs will tend to form as a stack of en-echelon fractures denoted as E-DITFs (Ito et al. 2001). The relationship between the induced fracture geometry and the in-situ stress orientations and magnitudes in this case is not as simple as in the aligned case (Brudy & Zoback 1993). Examples of A-DITFs and E-DITFs seen in the acoustic reflectivity images of the UBI log from BS1 are shown in Figs. 5a–b. The principal difficulty when interpreting borehole images for DITFs is to distinguish them from natural fractures. In the absence of core, or deep-probing, azimuthally-focussed electric logs such as the ARI (Luthi 2001), usage was made of the tendency of A-DITFs to have slightly zigzagging trace due to fracture linkage (Ito et al. 2001). E-DITFs were distinguished from stacks of parallel natural fractures by the tendency for their trace on the images to be weak or absent near the crests of each ‘sinusoid’ (Figure 5b). This is in contrast to natural fractures where the crests tend to be the most strongly defined part of the trace.

The orientation of each limb of each A-DITFs with respect to the borehole centre was measured every 40 cm. Thus, long, continuous A-DITFs have greater weight in averaging the measurements over the entire borehole length than short A-DITFs. For example, the orientation of a 4.8 meter long pair of diametrically-opposite A-DITFs will be described by 24 measurements, whereas a 2.4 m single-limbed A-DITFs will generate only 6 orientation measurements. The orientation of the E-DITFs was measured by fitting a sinusoid through the pair of limbs and determining the strike. Mean orientation is computed with each E-DITFs having the same weight.

Breakouts are typically manifest as pairs of diametrically-opposite spall zones that extend along the borehole axis. They occur when the local value of tangent compression at the borehole wall exceeds the compressive strength of the rock. In the simple case of a vertical borehole penetrating a rock mass in which one principal stress is vertical, breakouts, if they occur, develop where the tangent stress is a maximum, which occurs in the direction of the minimum horizontal principal stress,

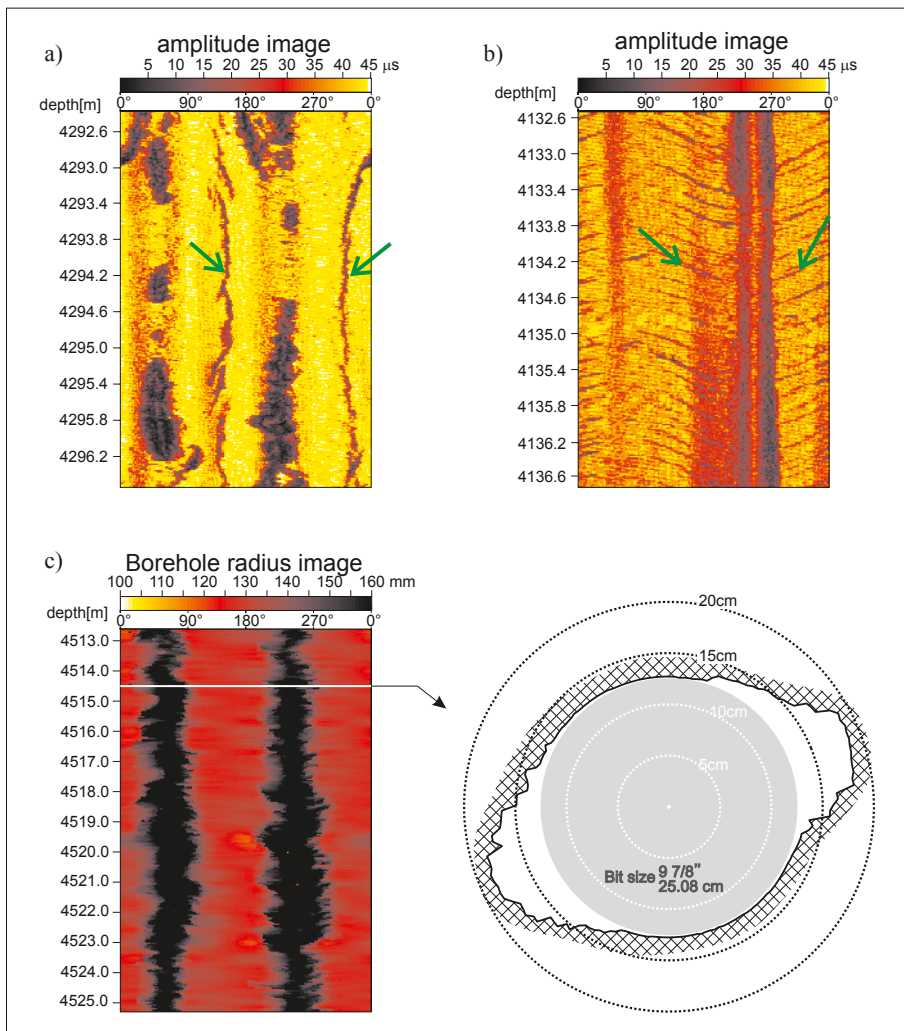


Fig. 5. a) Example of A-DITFs on BS1 images (green arrows). b) Example of E-DITFs on BS1 images (green arrows). c) Example of breakouts on the BS1 image (left) and the borehole cross-section computed from transit time (right).

Shmin. Breakouts affect the shape of the borehole cross-section significantly, as can be seen in Fig. 5c taken from the travel time image of BS1. Stress-induced breakouts are denoted by the darker bands (i.e. larger transit time) on the transit-time log of Fig. 5c. The identification of breakouts was performed using a quality ranking as follows: 'High-confidence' recognitions had two approximately-symmetric spall zones, and a difference between the minimum and maximum borehole radius greater than 2.5 cm; 'moderately-confidence' breakouts differed in that the difference between the minimum and maximum borehole radius was less than 2.5 cm; and 'low-confidence' were characterised by an asymmetric cross-section with one relatively deep narrow furrow and one poorly-developed enlargement diametrically-opposite. Isolated occurrences of the latter could be key-seats. However, in the case of BS1, such sections were usually continuous with 'high-confidence' breakouts, suggesting that most reflected stress-induced spalling. Other types of borehole enlargement, as for example washouts (Plumb & Hickman 1985) which are characterised by a borehole diameter increase in all directions, were excluded from the stress induced failure database. The breakout analysis was similar to the A-DITF analysis: the orientation of each limb of each breakout was determined every 40 cm, each measurement representing the orientation of the centre of the spall zone with respect to the borehole centre. Mean orientations were computed using the same procedure employed for A-DITFs. In addition to this visual identification of breakouts, an automatic examination of borehole cross-section shape was performed which involved the fitting of an ellipse to the borehole cross-section every 1 cm. The procedure used a direct least square fit as describe in Halir & Flusser (1998) to obtain the magnitude and direction of the minor and major axis. Note that such an approach is not able to distinguish between stress induced failure and borehole elongation due to other mechanisms, such as key-seats and washouts.

Results

BS1 borehole

Breakouts

The results of the automatic borehole cross-section shape analysis are presented first. The profiles of minor and major axis lengths of the fitted ellipses are shown in Fig. 3d and the corresponding orientation of the major axis in Fig. 3e. Sections with differences between major and minor axes greater than 2 cm, shown in red on Fig. 3e, are common. In such sections, the orientation of the major ellipse axis is very stable and has a mean orientation and standard deviation of $51^\circ \pm 13^\circ$ measured clockwise from true north.

For the 'manual' analysis, about 6,500 borehole cross-sections were analysed and 9,319 orientation measurements made. Some 1954 m or 81% of the logged section contained features considered to be potential stress-induced breakouts. In all,

'high-confidence' breakouts occurred along 1369 m of borehole, 'reasonably-confident' breakouts along 457 m and 'low-confidence' breakouts along 128 m. The results of the analysis are presented in Figs. 6a–b. The breakouts show a consistent preferred orientation along the entire borehole section which, for all three classes taken together, is $54 \pm 15^\circ$ (Fig. 6g). The 'high-confidence' breakouts have a preferred orientation of $52 \pm 14^\circ$, and the 'moderately-confident' breakouts present a statistically identical orientation of $54 \pm 14^\circ$. However, the 'low-confidence' breakout population show a significantly different mean breakout orientation of $71 \pm 14^\circ$, suggesting that some reflect mechanisms unrelated to stress-induced spalling.

Drilling-induced tension fractures

The A-DITFs and E-DITFs were analysed separately. In all, 245 m (10%) of the logged section in BS1 was affected by A-DITFs. The results are presented on Figs. 6c–d. The mean strike direction of A-DITFs is $151 \pm 13^\circ$ (Fig. 6h).

E-DITFs are observed along 314 m of borehole (13%). The en-echelon features, like the A-DITFs, are almost certainly tension fractures that are promoted by cooling during the drilling process. However, the geometry of the individual fractures and their spatial relationship to each other suggested mechanisms of their formation that are not adequately understood. Consequently, these were considered to be less reliable indicators of stress orientation than A-DITFs and were excluded from the computation of the statistics of our stress orientation estimates. They have a mean strike direction of $144 \pm 17^\circ$, with 82% dipping to the SW (see Figs. 6e–f). Their strike direction is thus consistent with the strike direction of A-DITFs, supporting the view that they are stress induced.

OT2 borehole

Breakouts

The presumed failure of the CBIL tool to register a reflection from azimuthal sectors of the hole that had suffered spalling means that the geometry of the spall is unknown. Thus, for these data we were unable to perform an automatic analysis by fitting ellipses, nor rank the certainty of the breakout identification. The azimuthal extent of the breakouts was taken as defined by the sectors of borehole enlargement indicated by either longer travel-times or anomalously-short travel times and low amplitudes, as discussed earlier (see Figs. 4 a–b). The orientation of the breakouts was determined in the same way as for the BS1 breakouts (Figs. 4 c–d). Breakouts occurred for 74 m or 37% of the analysed borehole length, mostly in the sedimentary section where 69 m or 70% was affected. In contrast, only 5 m or 5% of the basement section suffered breakouts. The mean orientation of breakouts is shown in the histograms of Fig. 7 and is $23 \pm 11^\circ$ in the sedimentary section and $62 \pm 14^\circ$ in the basement. Note that the relatively small number of measurement compared to BS1 produce relatively small standard deviations.

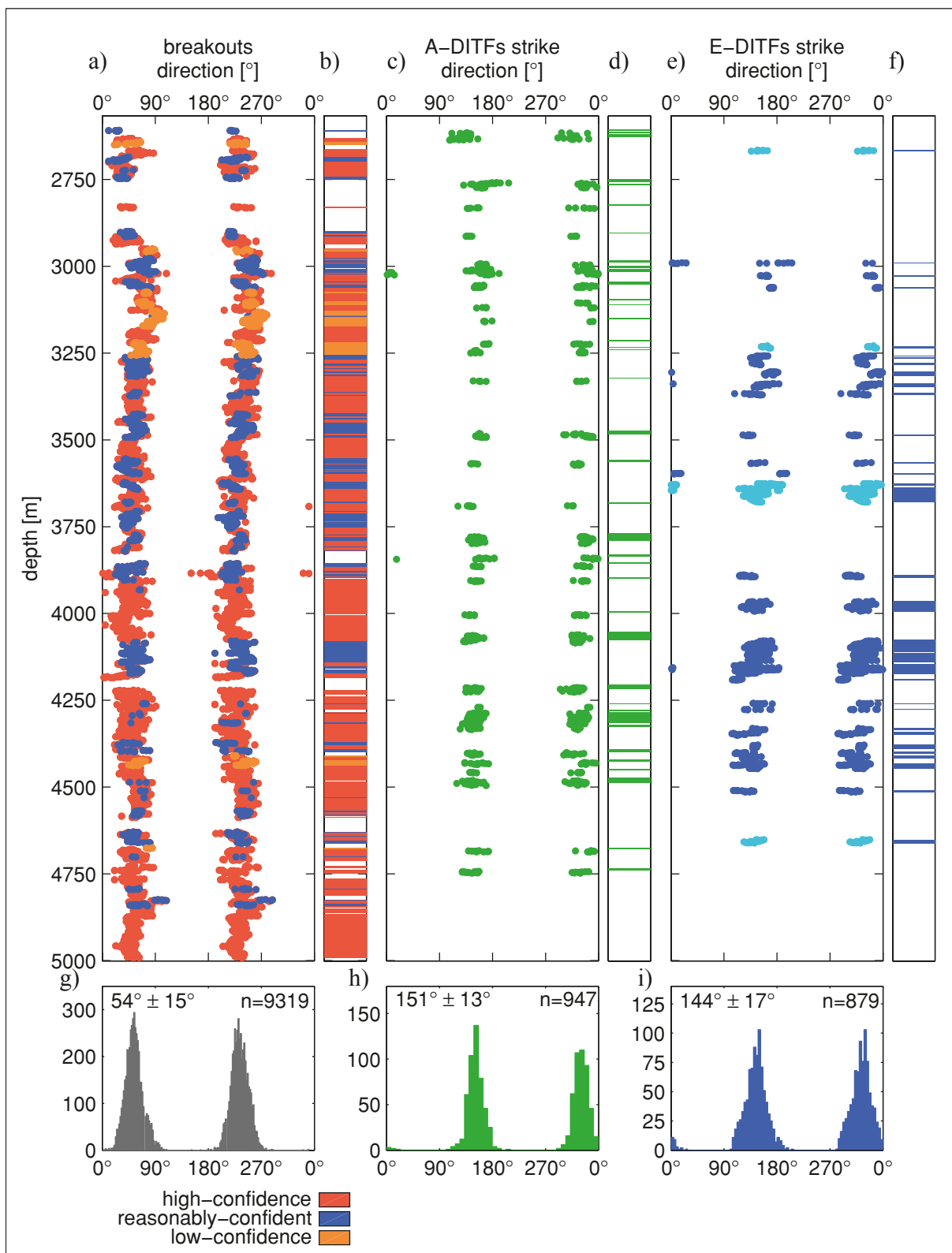


Fig. 6. Summary of stress induced wellbore failure in BS1. From left to right: a) Breakouts orientation determinations. b) Breakouts intervals with rating of confidence level. c) A-DITFs orientation measurements. d) A-DITF intervals. e) E-DITF measurements. Dark blue denotes features dipping to the SW and light blue features dipping to the NE. f) E-DITF intervals. g) Histogram of breakouts orientation. h) Histogram of A-DITFs orientation. i) Histogram of E-DITFs strike direction.

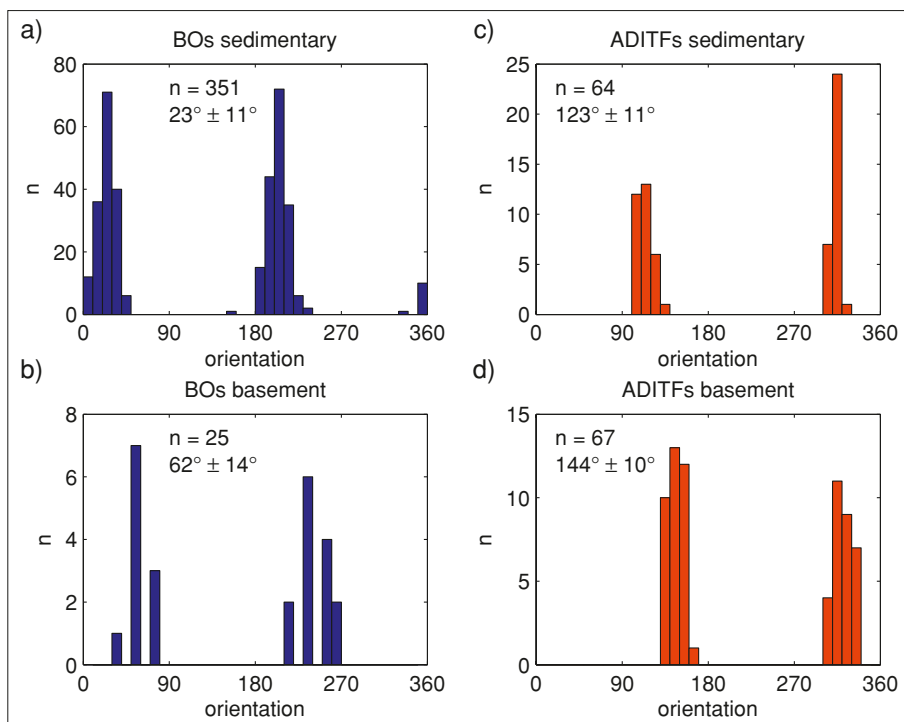


Fig. 7. Stress orientation indicators for the OT-2 borehole: a) Breakouts in the sedimentary section. b) Breakouts in the granite section. c) A-DITFs in the sedimentary section. d) A-DITFs in the granite section.

Drilling-induced tension fractures

A-DITFs were found in both the sediment and basement sections (Fig. 4c–d). A-DITFs occur in two clusters in the lowermost ~100 m of sediments where they have a cumulative length of 14 m and an orientation of $123 \pm 11^\circ$ (Fig. 7c). In the 100 m of basement, A-DITFs were again clustered, with cumulative length of 14 m, and have an orientation of $144 \pm 10^\circ$ (Fig. 7d).

Discussion

The boreholes are sufficiently close to vertical to permit the orientation of breakouts and DITFs to be taken as direct indicators of the orientation of the principal horizontal stresses.

The deviation from verticality of BS1 is less than 4° above 3600, less than 6° above 4600 m and never exceeds 8° , the azimuth varying between 270° and 360° . The OT2 deviation from vertical is 7° towards N 300° E for the depth of interest. The orientations of SHmax derived from the analyses of breakouts and A-DITFs for both holes are listed in Table 2 and shown in Fig. 8. ‘Low-confidence’ breakouts and E-DITFs were not included for the reasons mentioned earlier. The breakouts in BS1 indicate a mean SHmax orientation in the granite of $143 \pm 14^\circ$ and the A-DITFs indicate $151 \pm 13^\circ$. Combining both types of indicator gives a mean orientation for SHmax of $144 \pm 14^\circ$. We consider this to be the definitive result for mean SHmax orientation of the principal horizontal stresses.

Table 2. Summary of stress orientation determination.

Borehole	Coordinates	Collar altitude	Lithology/Failure type		Number of distinct zones	Combined failure length (% of failed borehole)	SHmax orient.	Quality ranking after WSW scheme
OT2	Swiss grid: 612 424 / 269 684 WGS84: 7° 36' 13.55" E 47° 34' 39.86" N	253 m a. s. l.	Sediment	A-DITFs	2	14 m (14%)	$123 \pm 11^\circ$	E
				breakouts	5	69 m (70%)	$113 \pm 11^\circ$	C
			Basement	A-DITFs	3	14 m (14%)	$144 \pm 10^\circ$	E
				breakouts	4	5 m (5%)	$152 \pm 14^\circ$	D
BS1	Swiss grid: 611'808 / 270'531 WGS84: 7° 35' 44.16" E 47° 35' 07.32" N	250 m a. s. l.	Basement	A-DITFs	38	245 m (10%)	$151 \pm 13^\circ$	B
				breakouts	182	1954 m (81%)	$143 \pm 14^\circ$	B

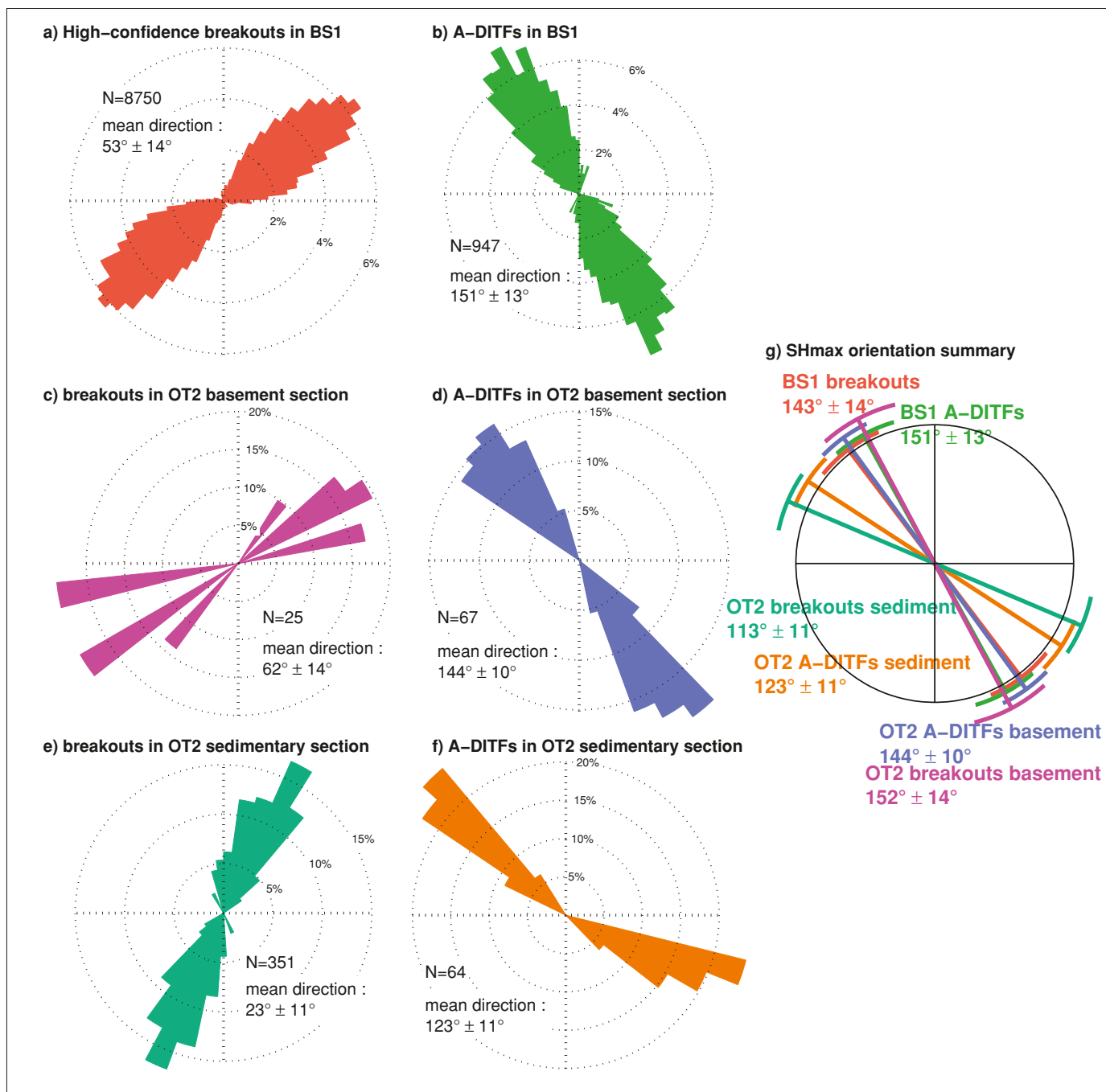


Fig. 8. Summary of the orientation of reliable stress indicators. a) High-confidence breakouts in BS1. b) A-DITFs in BS1. c) Breakouts in the basement section of OT2 d) A-DITFs in the basement section of OT2. e) Breakouts in the sedimentary section of OT2 f) A-DITFs in the sedimentary section of OT2. g) SHmax orientation (mean orientation and single circular standard deviation) derived from these indicators.

tation in the granite from 2600 m to 5000 m, and note it is consistent with the estimate for the uppermost 100 m of the granite obtained from the less-numerous breakouts and A-DITFs in OT2 of $152 \pm 14^\circ$ and $144 \pm 10^\circ$, respectively. The orientation of SHmax is very stable within the basement when averaged over scales of several hundred meters, although more-or-less

continuous variations are seen at shorter scales. These probably reflect stress heterogeneity related to stress perturbations at fractures, the analysis of which is beyond the scope of this paper.

The primary purpose of including the results from OT2 in the analysis was to examine SHmax orientation near the sedi-

ment interface. In the lowermost 100 m of the sedimentary section, A-DITFs have a cumulative length of 14 m and indicate a mean SHmax orientation of $123 \pm 11^\circ$, whereas breakouts extend over 69 m and suggest an SHmax orientation of $113 \pm 11^\circ$. Comparison with the results from DITFs and breakouts in the granite of $N144^\circ$ and 152° respectively suggest an $\sim 30^\circ$ change in SHmax orientation occurs near the basement interface. Whilst the number of data in the sedimentary section is relatively limited compared to the large number of data collected in the basement in the borehole BS1, which probably explains the 17° asymmetry of the mean orientation of the two DITF lobes in Fig. 8f, the orientation change is larger than can reasonably be attributed to noise. It should be noted that both breakouts and DITFs in the lowermost part of the sedimentary section were imaged in the same logging run used for the basement section, and thus the discrepancy in orientations cannot be attributed to changes in the sonde configuration. In view of this, it is concluded that a small but significant change in SHmax orientation occurs at the basement-sediment interface.

The origin of the change in stress at the basement-sediment interface is unclear. There is no evidence of a fracture zone or fault cutting the well near the interface. Since the latter constitutes an unconformity between the Rotliegende sediment and the weathered upper part of the granite, a contrast in mechanical properties between the sediment and intact granite can be expected which extends across the weathered zone. Stiffness contrast between beds subject to strain can produce bed-to-bed changes in stress magnitude and direction as shown by the field observations and modelling results of Bruno and Winterstein (1994) and Gudmundsson (2006, 2009). However, if the deformation is uniform horizontal straining, then the development of changes in horizontal stress orientation between beds of different elastic stiffness requires some obliquity of the beds to the horizontal plane (Bruno and Winterstein, 1994), or the presence of elastic anisotropy in the horizontal plane. Although we cannot rule these out, the available information provides no indication of strong horizontal elastic anisotropy of the bulk rock elastic properties, and the topography of the granite surface is believed to be mild with a large-scale dip of $\sim 12^\circ$ to the east, which largely accounts for the different depths to the granite in the two wells (the wellhead heights above sea level differ by only 2 m – see Fig. 2). Another possible explanation is that the change in stress orientation reflects the presence of remnant stresses. Immediately prior to the deposition of the Rotliegende, some stress state appropriate for an erosional surface existed in the crystalline basement, and it is more probable than not that the stresses were anisotropic in the horizontal plane. This stress state would not be present in the sedimentary rocks that were subsequently deposited, and hence a contrast in horizontal stress would be present. However, since the granite was at the surface at the onset of deposition, the stress magnitudes near the interface would be relatively small in comparison to subsequent modification from burial and loading, and it is questionable whether they could explain $20\text{--}30^\circ$ of rotation of SHmax at the interface today. Thus, the origin of the stress

orientation change in the OT2 stress profile at the interface is uncertain. It is important to note that the present data do not define whether it is a local or extensive feature. Nevertheless, the practical consequence of a $20\text{--}30^\circ$ error in stress orientation for the design and development of an EGS in the basement would be significant, and serve to emphasise the importance of obtaining stress information within the target reservoir and not extrapolating from shallower measurements.

The quality ranking of the different stress estimates following the ranking scheme of the World Stress Map project (Sperner et al. 2003) is given in Table 2. Under this scheme, the results from OT2 would be ranked as questionable (D) or not reliable (E) except for the breakouts in the sedimentary section which would be ranked as ‘C’ (i.e. standard deviation in the range $20\text{--}25^\circ$). The stress orientation estimates from the A-DITFs and breakouts in BS1 would both be ranked as ‘B’ quality, despite stemming from almost 2 km of data. We believe the failure to rank the BS1 data as ‘A’ reflects limitations in the WSM quality ranking scheme in deep boreholes with pervasive borehole failure, rather than deficiencies in the data. Under the WSM scheme, a ‘B’ quality is mandated because the standard deviation is 14° rather than $\leq 12^\circ$ as required for ‘A’ quality estimates. However, the standard deviation reflects true stress heterogeneities in the basement that only long profiles can capture. We note that other long profiles of SHmax orientation from wellbore failure at Soultz (Valley & Evans 2010) and the KTB (Brudy & Zoback 1999) also rank as ‘B’ for the same reasons. Such information is valuable and should not lead to degradation in the quality of the indicator.

The relation of the mean SHmax orientation from borehole BS1 to other basement stress indicators in the region about Basel is shown in Fig. 9d and e. Most estimates are from breakouts, with some hydrofracture and overcoring measurements. The areas labelled F3 and F4 denote regions of uniform stress defined from rigorous inversion of fault plane solutions (FPS) of earthquakes that lie within the boundaries drawn on the figure (Kastrup et al. 2000). Most events lie at depths of $10\text{--}15$ km, with some extending to 20 km depth for area F3 of the alpine foreland. The mean orientation of Shmin for both regions was found to be $N55^\circ E$. Deichmann and Ernst (2009, this volume) derived fault plane solutions for the 28 strongest events induced within the rock mass around BS1 by the stimulation injections of 2007. They found the orientation of the T-axis of the events was sub-horizontal and oriented $N48^\circ E \pm 13^\circ$. Thus, with the exception of the overcoring measurements, all basement data indicate NW–SE oriented compression. The agreement of the mean Shmin orientation from BS1 with that indicated by inversion of the FPSs from the areas F3 and F4 to the south and north, and the T-axes of events within the rock volume around the well provides support for the validity of the relatively indirect stress-orientation estimates obtained from the FPSs of both natural and induced seismic events respectively.

The NW-orientation of SHmax is consistent with the average SHmax-orientation of the large-scale stress field of Western Europe north of the Swiss alpine foreland that is believed

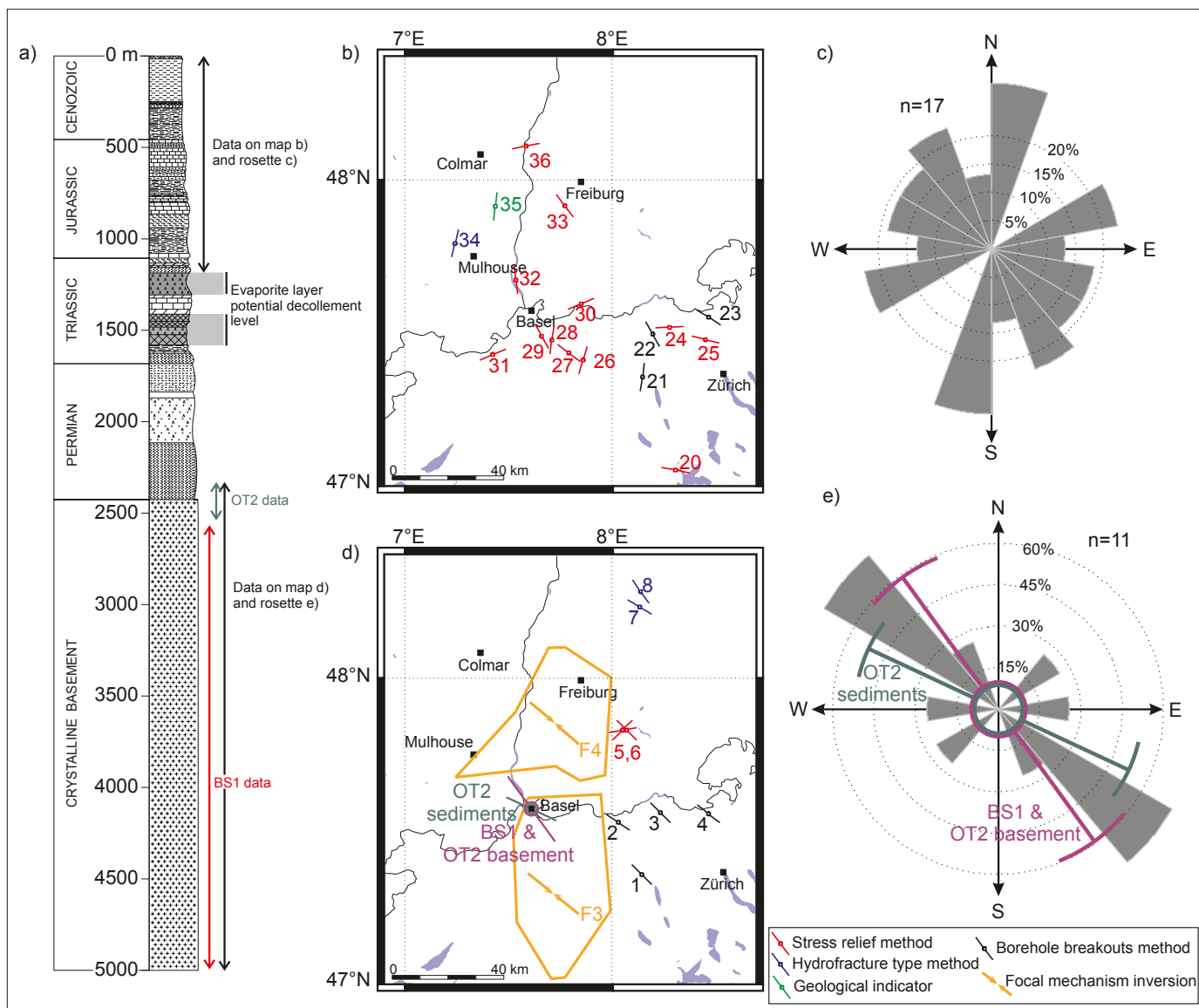


Fig. 9. Estimates of SHmax orientation for the Upper Rhine Graben and Swiss foreland region centred on Basel. a) Lithological section (see also Figure 2) showing the data catchment zones. The stress data presented here are either above the evaporate layers (decollement level), in the sediments below the evaporate layers or in the crystalline basement. b) and c) Estimates of SHmax orientation in the Mesozoic and younger section that lies within or above the evaporite layers (after Evans & Roth 1998). d) and e) Estimates of SHmax orientation in the crystalline basement derived from various borehole measurements and the inversion of focal mechanisms of earthquakes (the areas of uniform stress defined by the earthquake populations used in the inversions are outlined). The key to the datapoint numbers are as follows: 1/2/3/4 (Becker et al. 1987; Blümling 1986); 5/6 (Wolter 1987); 7 (Becker and Paladini, 1992; Rummel & Baumgaerner 1987); 8 (Baumgärtner & Zoback 1989); 9 Reinecker et al. (2005); 20 (Gysel 1975); 21/22/23 (Becker et al. 1987; Blümling 1986); 24/25 (Müller et al. 1987); 26/27/28/30 (Becker et al. 1987); 31 (Becker & Werner 1995); 32/33 (Greiner & Illies 1977); 34/35 (Cornet & Burlet 1992); 36 (Leopoldt 1979).

to be related to the push from the North Atlantic Ridge and to the convergence between Africa and Europe (Müller et al. 1992). We see no evidence of a rotation of SHmax towards a more NNW- to N-orientation below 3 km, as tentatively proposed by Cornet et al. (2007) to be a characteristic of stress in north-western Europe.

The oriented logs run in BS1 and OT2 do not extend higher than 80 m above the basement interface, and so the data do not allow the question of whether a further change in stress

orientation occurs at the evaporites, as reported for measurements conducted elsewhere in the Swiss foreland basin (Müller et al. 1987). Extensive salt detachments often constitute stress discontinuity horizons (Davis & Engelder 1985): for example, Evans (1989) found a discontinuity in horizontal stress magnitude across the Salinian salt of the Appalachian basin, and Röckel and Lempp (2003) report a discontinuity in horizontal stress magnitude and direction at the Zechstein salt in the North German basin. It is thus of interest to compare

the SHmax-orientation results from the two boreholes with those from near-surface stress measurements conducted above the evaporite horizon in the vicinity of Basel. These data are shown in Fig. 9b. Most derive from overcoring or borehole slotter measurements conducted at shallow depth in quarries or tunnels, although they also include hydrofracture orientation data from relatively large-volume injections into Muschelkalk beds at depths of 550–850 m at a potash mine, and an active salt diapir (Cornet & Burlet 1992: data points 34 and 35 resp. in Fig. 9b). The scatter in the collective data is much greater than seen in the generally deeper crystalline measurements, reflecting the highly-heterogeneous nature of the near-surface state of stress. If the heterogeneity reflects random fluctuations in an otherwise uniform stress field, then averaging can be applied to resolve the underlying uniform trend. The average SHmax orientation of the 7 data points taken within 50 km of Basel is $N177^{\circ}E \pm 66^{\circ}$ (note this is not strictly the best standard deviation estimate since the error associated with each individual data point was not included). The mean value of $N177^{\circ}E$ differs markedly from the basement orientation of $N144^{\circ}E \pm 14^{\circ}$, although whether this reflects a systematic difference in mean SHmax orientation above or below the evaporites is not strictly resolved: for even if the conditions required to render averaging sensible are satisfied, the standard deviation is so large as to require more data points in order to robustly distinguish between N–S and NW–SE orientations of SHmax. Nevertheless, the shallower data do favour a more N–S orientation, and the deeper hydrofracture data and an active salt diapir (Cornet & Burlet, 1992): data point 35 in Fig. 9b) support the assertion that the stress field above the evaporites in the Rhine Graben within 50 km of Basel is on average oriented N–S, and that a change in SHmax-orientation occurs at the evaporite horizon. Thus, in the section studied, there is at least one and possibly two horizons where a discontinuity in SHmax-orientation occurs. For deep geothermal energy development, this result emphasises the need to determine the stress state in the target reservoir rock rather than relying on extrapolation of data from shallower horizons.

Conclusions

Analysis of the orientation of breakouts and drilling-induced tension fractures in two deep boreholes penetrating the crystalline basement under the city of Basel indicates a mean orientation of SHmax within the granite from near the sediment interface down to 5 km depth is $N144^{\circ}E \pm 14^{\circ}$. The orientation of SHmax is very stable within the basement when averaged over scales of several hundred meters, and is consistent with the results of analyses of the focal mechanism solutions of natural regional earthquakes and also events induced during the injections. DITFs and breakouts in the Rotliegend sandstone that lies immediately above the basement indicate an SHmax orientation of $N115^{\circ}E \pm 12^{\circ}$, suggesting a $\sim 30^{\circ}$ change in SHmax-orientation occurs near the basement-sediment interface. The origin of this change is uncertain. The logs do not

extend higher than 80 m above the interface, and so the profile of SHmax orientation does not extend through the evaporites. Near-surface measurements within 50 km of Basel suggest a mean orientation of N–S, although the variability is very large. Nevertheless, an average N–S orientation for SHmax above the evaporites within 50 km of Basel is also suggested by hydrofractures induced at up to 850 m depth and a salt diapir. Thus, the balance of evidence suggests an SHmax orientation change also occurs at the evaporite horizon. Changes in stress orientation can have significant practical consequences on the design and development of an EGS, and serve to emphasise the importance of obtaining estimates within the target rock mass.

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